

ΔΔ4b1 BARS TO BE EPOXY COATED.

\*\* WHERE DEFLECTING STRANDS INTERFERE WITH PLACEMENT, SOME IN-PLACE BENDING MAY BE NECESSARY.

	REINFORCING BAR LIST												88 88
	BEAN	SPAN	B50	50′-10	B55	55′-0	B59	59′-2	B63	63'-4	B67	67′-6	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2
	BAR	SHAPE	NO.	LENGTH	0=2 3'-5 <sup>3</sup> / <sub>4</sub> 3'-6 <sup>3</sup> / <sub>4</sub>								
	6al	—	4	27′-3	4	29′-4	4	31′-5	4	33′-6	4	35′-7	D=14
	4a2		2	4′-2	2	4′-2	2	4′-2	2	4′-2	2	4′-2	1'-  4b2
ΔΔ	4b1	$\Box$	44	7′-10	46	7′-10	50	7′-10	52	7′-10	56	7′-10	ΔΔ4b1 4½ 3d
	4b2		12	6′-2	12	6′-2	12	6′-2	12	6′-2	12	6′-2	· · ·
	4b5		—	—	—	—	—	—	12	3′-3	12	3′-3	1'-1½ 3e
	3cl		44	1′-5	46	1′-5	50	1′-5	52	1′-5	56	1′-5	D=13 M# -# /20
**	3d	2	112	2′-10	116	2′-10	124	2′-10	128	2′-10	136	2′-10	D=1½ mt 22 11 10½ 3ci
	3e		24	1′-8	24	1′-8	24	1′-8	24	1′-8	24	1′-8	3'-1
													ALL DIMENSIONS ARE
													OUT TO OUT.  RADIUS TO © BAR.
													D = PIN DIAMETER.

## B BEAM DATA STRAIGHT LY OF THE STREET OF THE STREES OF THE STREES OF THE STREET OF T BEAM (L) STRAND SIZE DIA. (inches) CAMBER (in.) DEFLECTION (in.) 40 PERMISSIBLE SPACING WEIGHT SPAN LENGT CONCRETE (C. Y.) (TONS) IMMEDIATE TIME (ELASTIC) AT OVERALL **AFTER** HI 93 BEAM RELEASE LOSSES LOADING ₩, CONC. STEEL CONC. STEEL CONC. STEEL DIAPH, DIAPH, DIAPH, DIAPH DIAPH DIAPH B50 50'-10 51'-10 0.60 8 2 425 10.8 0.67 1.24 0.43 0.39 0.11 0.10 7'-6 7'-6 10.3 5.10 607 \*B55 55'-0 56'-0 0.60 8 3 468 I4.I 0.85 0.58 0.54 0.14 0.13 7'-6 7'-6 11.2 1.51 5.51 635 7'-6 7'-6 12.0 \*B59 59'-2 60'-2 0.60 IO 3 554 I3.2 I.I2 1.99 0.82 0.77 0.21 0.19 5.92 680 \*B63 63'-4 64'-4 0.60 12 3 639 12.3 1.30 7'-6 7'-6 12.8 2.32 0.91 0.84 0.23 0.21 6.33 733 \*B67 67'-6 68'-6 0.60 14 3 724 11.6 1.69 2.98 1.16 1.09 0.29 0.27 7'-6 7'-6 13.6 6.74 778

DEFLECTIONS AT MID-SPAN DUE TO WEIGHT OF SLAB AND DIAPHRAGM. THE DEFLECTIONS SHOWN ARE FOR A SLAB WEIGHT OF 757 #/FT. (8" SLAB AND 7"-6 BEAM SPACING) AND ONE CONCRETE DIAPHRAGM (2270 #) OR ONE STEEL DIAPHRAGM (2285 #) AT © OF SPAN. FOR DIFFERENT SLAB AND DIAPHRAGM WEIGHTS, DEFLECTIONS WILL BE DIRECTLY PROPORTIONAL.

@ DEFLECTIONS DUE TO THE COMBINED EFFECT OF CREEP DUE TO WEIGHT OF SLAB AND SHRINKAGE OF SLAB.

TOTAL BEAM DEFLECTIONS AT  $\P$  OF SPAN,  $\Delta_D$ , DUE TO WEIGHT OF SLAB AND DIAPHRAGMS FOR DETAILING PURPOSE: (A)  $\Delta_D = \Delta_1 + \Delta_T$  FOR SIMPLE SPAN. (B)  $\Delta_D = \Delta_1 + \Delta_T$  FOR END SPANS OF CONTINUOUS BRIDGE.

(B)  $\Delta_D$  =  $\Delta_1 + \frac{3}{4}\Delta_T$  FOR END SPANS OF CONTINUOUS BRIDGE. (C)  $\Delta_D$  =  $\Delta_1 + \frac{1}{2}\Delta_T$  FOR INTERIOR SPANS OF CONTINUOUS BRIDGE.

 $\ensuremath{\mathfrak{D}}$  TOTAL INITIAL PRESTRESS IS BASED ON 72.6% f's, f's = 270 ksi AND As = 0.217 sq. in.

\* MINIMUM CONCRETE f'c (AT 28 DAYS) SHALL BE 7,000 psi. MINIMUM f'ci AT RELEASE SHALL BE 6,000 psi.

## SPECIFICATIONS:

CONSTRUCTION: STANDARD SPECIFICATIONS OF THE IOWA
DEPARTMENT OF TRANSPORTATION, CURRENT SERIES, WITH
CURRENT APPLICABLE SPECIAL PROVISIONS AND SUPPLEMENTAL SPECIFICATIONS.

DESIGN: A.A.S.H.T.O. LRFD, SERIES OF 2007, WITH MINOR MODIFICATIONS.

## **DESIGN STRESSES:**

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE TO BE IN ACCORDANCE WITH A.A.S.H.T.O. LRFD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2007:

REINFORCING STEEL IN ACCORDANCE WITH SECTION 5, GRADE 60.

CONCRETE IN ACCORDANCE WITH SECTION 5, f'c = 5000 psi
(FXCEPT AS NOTED)

PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 5, f's = 270,000 psi.

## NOTES:

THESE BEAMS ARE DESIGNED FOR AASHTO LIVE LOADS AS INDICATED IN ABOVE TABLE WITH AN ALLOWANCE OF 20 Ib. PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE. HOLD DOWN POINTS FOR DEFLECTED STRANDS MAY BE

MOVED TOWARD ENDS OF BEAM A DISTANCE OF 0.05 L MAXIMUM AT PRODUCER'S OPTION.

ALL PRESTRESSING STRANDS SHALL CONFORM TO ASTM A416

ALL PRESTRESSING STRANDS SHALL CONFORM TO ASTM A41 GRADE 270 LOW RELAXATION STRANDS.

TOPS OF BEAMS ARE TO BE STRUCK OFF LEVEL AND FINSHED AS PER MATERIALS IM570.

BEARINGS SHALL BE AS DETAILED ON OTHER DESIGN SHEETS. BEAMS TO BE USED IN BRIDGES MADE CONTINUOUS BY THE POURED IN PLACE FLOOR, ARE TO BE AT LEAST 28 DAYS OLD BEFORE THE FLOOR IS PLACED UNLESS A SHORTER CURING TIME IS APPROVED BY THE BRIDGE ENGINEER.

THE PORTIONS OF THE PRESTRESS BEAMS THAT ARE TO BE EMBEDDED IN THE ABUTHANT AND PIECE DIAPHRAGMS SHALL BE ROUGHENED FOR A DISTANCE OF 10" FROM THE BEAM END BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE BEAM AND THE DIAPHRAGM IN ACCORDANCE WITH ARTICLE 2403.03, I, OF THE STANDARD SPECIFICATIONS.

ALL BEAMS ARE TO BE INCREASED IN LENGTH TO COMPENSATE FOR ELASTIC SHORTENING, CREEP AND SHRINKAGE.

IF THE STEEL DIAPHRAGM OPTION IS ALLOWED AND USED,

IF THE STEEL DIAPHRAGM OPTION IS ALLOWED AND USED, HOLES MUST BE CAST IN THE WEB TO ACCOMMODATE THE STEEL DIAPHRAGM ATTACHMENTS AS DETAILED ON THE STEEL DIAPHRAGM DETAIL SHEET.

IF SOLE PLATE IS REQUIRED FOR BEARING, SOLE PLATE IS TO BE SET IN FORMS WHEN BEAM IS CAST AND FORMED OUT BELOW TO EXCLUDE CONCRETE AS DETAILED ON THE BEARING SHEET.

0.6" DIAMETER STRANDS STRESSED TO NOT MORE THAN 5,000 LBS. EACH MAY BE USED IN LIEU OF THE a BARS WHICH RUN THE FULL LENGTH OF THE BEAM IN THE TOP FLANGE.

