

ΔΔ4bI BARS TO BE EPOXY COATED.

BEAM

SPECIFICATION REFERENCES WERE

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60-01

** WHERE DEFLECTING STRANDS INTERFERE WITH PLACEMENT, SOME IN-PLACE BENDING MAY BE NECESSARY.

			F	REIN	FΟ	RCIN	1G	88 3	1					
	BEAN	SPAN	B50	50′-10	B55	55′-0	B59	59′-2	B63	63'-4	B67	67'-6	25.2	l
	BAR	SHAPE	NO.	LENGTH		l								
	6a1		4	27′-3	4	29'-4	4	31′-5	4	33′-6	4	35′-7	- N D=1 2	ı
	4a2	_	2	4'-2	2	4′-2	2	4'-2	2	4′-2	2	4′-2	1/ <u>1</u> 1/- 4b2	ı
ΔΔ	4b1		44	7′-10	46	7′-10	50	7′-10	52	7′-10	56	7′-10	ΔΔ4b1 4½ 3d	ı
	4b2		12	6′-2	12	6′-2	12	6′-2	12	6′-2	12	6′-2		ı
	4b5		—		—		—	—	12	3′-3	12	3′-3	1'-1½ 3e	ı
	3c1		44	1′-5	46	1′-5	50	1′-5	52	1′-5	56	1′-5	 	ı
**	3d	2	112	2′-10	116	2′-10	124	2′-10	128	2′-10	136	2′-10		ı
	3е		24	1′-8	24	1′-8	24	1′-8	24	1′-8	24	1′-8	10½ 3c1 3′-1	ı
													ALL DIMENSIONS ARE	ı
													OUT TO OUT. RADIUS TO & BAR.	ı
													D = PIN DIAMETER.	ı

B BEAM DATA

BEAM	SPAN LENGTH	OVERALL BEAM LENGTH (L)	STRAND SIZE DIA. (inches)	NO.OF STRANDS			NN S	CAMBER (in.)				DEFLECTION (in.) Δ _D				PERMISSIBLE SPACING			WEIGHT (TONS)))	CING	
				IGHT		TOTAL IN PRESTRE KIPS	음¥		AT RELEASE		AFTER LOSSES		IMMEDIATEÛ (ELASTIC) ∆ _I		TIME ② (PLASTIC) Δ _T				HL93 LOADING		(10143)		INFORCI TEEL-(II
				STRA	DEFLE							CONC. DIAPH.		CONC. DIAPH.	STEEL DIAPH.				STEEL DIAPH.			CONCRET (C. Y.)	RE
B50	50′-10	51′-10	0.60	8	2	425	10.8	0.67		1.24		0.43	0.39	0.11	0.10			7′-6	7′-6	10.3		5.10	607
≭ B55	55′-0	56′-0	0.60	8	3	468	14.1	0.85		1.51		0.58	0.54	0.14	0.13			7′-6	7′-6	11.2		5.51	635
≭ B59	59′-2	60′-2	0.60	10	3	554	13.2	1.12		1.99		0.82	0.77	0.21	0.19			7′-6	7′-6	12.0		5.92	680
* B63	63'-4	64'-4	0.60	12	3	639	12.3	1.30		2.32		0.91	0.84	0.23	0.21			7′-6	7′-6	12.8		6.33	733
 \$867	67'-6	68′-6	0.60	14	3	724	11.6	1.69		2.98		1.16	1.09	0.29	0.27			7′-6	7′-6	13.6		6.74	778

DEFLECTIONS AT MID-SPAN DUE TO WEIGHT OF SLAB AND DIAPHRAGM. THE DEFLECTIONS SHOWN ARE FOR A SLAB WEIGHT OF 757 #/FT.(8" SLAB AND 7'-6 BEAM SPACING) AND ONE CONCRETE DIAPHRAGM (2270 #) OR ONE STEEL DIAPHRAGM (285 #) AT & OF SPAN, FOR DIFFERENT SLAB AND DIAPHRAGM WEIGHTS, DEFLECTIONS WILL BE DIRECTLY PROPORTIONAL.

② DEFLECTIONS DUE TO THE COMBINED EFFECT OF CREEP DUE TO WEIGHT OF SLAB AND SHRINKAGE OF SLAB.

TOTAL BEAM DEFLECTIONS AT § OF SPAN, Δ_D , DUE TO WEIGHT OF SLAB AND DIAPHRAGMS FOR DETAILING PURPOSE:

(a) $\Delta_0 = \Delta_1 + \Delta_1$ FOR SIMPLE SPAN. (B) $\Delta_0 = \Delta_1 + \Delta_1$ FOR END SPANS OF CONTINUOUS BRIDGE. (C) $\Delta_0 = \Delta_1 + \frac{1}{2}\Delta_1$ FOR INTERIOR SPANS OF CONTINUOUS BRIDGE.

TOTAL INITIAL PRESTRESS IS BASED ON 72.6% f's, f's = 270 ksi AND As = 0.217 sq. in.

MINIMUM CONCRETE f'c (AT 28 DAYS) SHALL BE 7.000 psi. MINIMUM f'ci AT RELEASE SHALL BE 6,000 psi.

SPECIFICATIONS:

CONSTRUCTION: STANDARD SPECIFICATIONS OF THE IOWA
DEPARTMENT OF TRANSPORTATION. CURRENT SERIES. WITH CURRENT APPLICABLE SPECIAL PROVISIONS AND SUPPLE-MENTAL SPECIFICATIONS.

DESIGN: A.A.S.H.T.O. LRFD. SERIES OF 2007, WITH MINOR MODIFICATIONS.

DESIGN STRESSES:

DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE TO BE IN ACCORDANCE WITH A.A.S.H.T.O. LRFD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2007:

REINFORCING STEEL IN ACCORDANCE WITH SECTION 5. GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 5, f'c = 5000 psi (EXCEPT AS NOTED)

PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 5, f's = 270,000 psi.

NOTES:

THESE BEAMS ARE DESIGNED FOR AASHTO LIVE LOADS AS INDICATED IN ABOVE TABLE WITH AN ALLOWANCE OF 20 15. PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE. HOLD DOWN POINTS FOR DEFLECTED STRANDS MAY BE
MOVED TOWARD ENDS OF BEAM A DISTANCE OF 0.05 L MAXIMUM

AT PRODUCER'S OPTION. ALL PRESTRESSING STRANDS SHALL CONFORM TO ASTM A416 GRADE 270 LOW RELAXATION STRANDS.

TOPS OF BEAMS ARE TO BE STRUCK OFF LEVEL AND FINSHED AS PER MATERIALS IM570.

BEARINGS SHALL BE AS DETAILED ON OTHER DESIGN SHEETS.

BEAMS TO BE USED IN BRIDGES MADE CONTINUOUS BY
THE POURED IN PLACE FLOOR, ARE TO BE AT LEAST 28 DAYS OLD BEFORE THE FLOOR IS PLACED UNLESS A SHORTER CURING TIME IS APPROVED BY THE BRIDGE ENGINEER.
THE PORTIONS OF THE PRESTRESS BEAMS THAT ARE TO BE

EMBEDDED IN THE ABUTMENT AND PIER DIAPHRAGMS SHALL BE ROUGHENED FOR A DISTANCE OF 10" FROM THE BEAM END BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE BEAM AND THE DIAPHRAGM IN ACCORDANCE WITH ARTICLE 2403.03, I, OF THE STANDARD SPECIFICATIONS

ALL BEAMS ARE TO BE INCREASED IN LENGTH TO COMPENSATE FOR ELASTIC SHORTENING, CREEP AND SHRINKAGE.

IF THE STEEL DIAPHRAGM OPTION IS ALLOWED AND USED.

HOLES MUST BE CAST IN THE WEB TO ACCOMMODATE THE STEEL DIAPHRAGM ATTACHMENTS AS DETAILED ON THE STEEL DIAPHRAGM DETAIL SHEET.

IF SOLE PLATE IS REQUIRED FOR BEARING, SOLE PLATE IS TO BE SET IN FORMS WHEN BEAM IS CAST AND FORMED OUT BELOW TO EXCLUDE CONCRETE AS DETAILED ON THE BEARING SHEET. 0.6" DIAMETER STRANDS STRESSED TO NOT MORE THAN 5,000

LBS. EACH MAY BE USED IN LIEU OF THE a BARS WHICH RUN THE FULL LENGTH OF THE BEAM IN THE TOP FLANGE.

10-09 REVISION DATE 1/√ 2/2 LATEST

lowa Department of Transportation **Highway Division**

STANDARD DESIGN - 40' ROADWAY, THREE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES AUGUST, 2009

B BEAM DETAILS

H40-34-06