

	A BEAM DATA																						
ВЕАМ	SPAN LENGTH	BE AM	Sıze	NO.	OF ANDS	TOTAL INITIAL PRESTRESS © KIPS	HOLD DOWN FORCE-KIPS	CAMBER (in.)				DEFLECTION (in.) Δ _D			PERMISSIBLE SPACING			WEIGHT (TONS)		ш	CING		
		VERALL F		IGHT						AFT LOS		IMMEDIATEÛ (ELASTIC) ∆ _I		TIME ② (PLASTIC) A _T				HL93 LOADING		(10143)		CONCRETE (C. Y.)	INFORCI TEEL-(II
		OVE!	STR	STRA	DEFLE									CONC. DIAPH.					STEEL DIAPH.			Ю	RE S
A42	42′-6	43′-6	0.6"	7	2	383	9.3	0.70		1.24		0.38	0.35	0.09	0.09			7′-6	7′-6	7.1		3.49	452
* A46	46′-8	47′-8	0.6"	8	2	426	8.5	0.76		1.35		0.50	0.47	0.13	0.12			7′-6	7′-6	7.7		3.82	488
*A50	50′-10	51′-10	0.6"	9	3	510.9	11.7	1.02		1.82		0.69	0.65	0.17	0.16			7′-6	7′-6	8.4		4.15	503
*A55	55′-0	56′-0	0.6"	10	3	553.4	10.8	1.29		2.30		0.94	0.88	0.23	0.22			7'-6	7′-6	9.1		4.49	547

DEFLECTIONS AT MID-SPAN DUE TO WEIGHT OF SLAB AND DIAPHRAGM. THE DEFLECTIONS SHOWN ARE FOR A SLAB WEIGHT OF 757 #/FT.(8° SLAB AND 7'-6 BEAM SPACING) AND ONE CONCRETE DIAPHRAGM (1912 #) OR ONE STEEL DIAPHRAGM (2912 #) AT § OF SPAN. FOR DIFFERENT SLAB AND DIAPHRAGM WEIGHTS, DEFLECTIONS WILL BE DIRECTLY PROPORTIONAL.

② DEFLECTIONS DUE TO THE COMBINED EFFECT OF CREEP DUE TO WEIGHT OF SLAB AND SHRINKAGE OF SLAB.

TOTAL BEAM DEFLECTIONS AT \P OF SPAN, Δ_0 , DUE TO WEIGHT OF SLAB AND DIAPHRAGMS FOR DETAILING PURPOSE: (A) $\Delta_D = \Delta_1 + \Delta_T$ FOR SIMPLE SPAN.

(B) $\Delta_0 = \Delta_1 + \frac{3}{4}\Delta_T$ FOR END SPANS OF CONTINUOUS BRIDGE. (C) $\Delta_D = \Delta_1 + \frac{1}{2}\Delta_T$ FOR INTERIOR SPANS OF CONTINUOUS BRIDGE.

3 TOTAL INITIAL PRESTRESS IS BASED ON 72.6% f's, f's = 270 ksi AND As = 0.217 sq. in.

* MINIMUM CONCRETE f'c (AT 28 DAYS) SHALL BE 7,000 psi.
MINIMUM f'ci AT RELEASE SHALL BE 6,000 psi.

SPECIFICATIONS:

CONSTRUCTION: STANDARD SPECIFICATIONS OF THE IOWA DEPARTMENT OF TRANSPORTATION, CURRENT SERIES, WITH CURRENT APPLICABLE SPECIAL PROVISIONS AND SUPPLE-MENTAL SPECIFICATIONS.

DESIGN: A.A.S.H.T.O. LRFD, SERIES OF 2007, WITH MINOR MODIFICATIONS.

DESIGN STRESSES:

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DESIGN STRESSES FOR THE FOLLOWING MATERIALS ARE TO BE IN ACCORDANCE WITH A.A.S.H.T.O. LRFD SPECIFICATIONS FOR HIGHWAY BRIDGES, SERIES OF 2007:

REINFORCING STEEL IN ACCORDANCE WITH SECTION 5, GRADE 60. CONCRETE IN ACCORDANCE WITH SECTION 5, f'c = 5000 psi (EXCEPT AS NOTED)

PRESTRESSING STEEL IN ACCORDANCE WITH SECTION 5. f's = 270,000 psi.

NOTES:

THESE BEAMS ARE DESIGNED FOR AASHTO LIVE LOADS AS INDICATED IN ABOVE TABLE WITH AN ALLOWANCE OF 20 16.PER SQUARE FOOT OF ROADWAY FOR FUTURE WEARING SURFACE. HOLD DOWN POINTS FOR DEFLECTED STRANDS MAY BE

MOVED TOWARD ENDS OF BEAM A DISTANCE OF 0.05 L MAXIMUM AT PRODUCER'S OPTION.

ALL PRESTRESSING STRANDS SHALL CONFORM TO ASTM A416 GRADE 270 LOW RELAXATION STRANDS.
TOPS OF BEAMS ARE TO BE STRUCK OFF LEVEL AND

FINSHED AS PER MATERIALS IM570.

BEARINGS SHALL BE AS DETAILED ON OTHER DESIGN SHEETS. BEAMS TO BE USED IN BRIGGES MADE CONTINUOUS BY
THE POURED IN PLACE FLOOR, ARE TO BE AT LEAST 28 DAYS OLD
BEFORE THE FLOOR IS PLACED UNLESS A SHORTER CURING TIME IS APPROVED BY THE BRIDGE ENGINEER.

THE PORTIONS OF THE PRESTRESS BEAMS THAT ARE TO BE EMBEDDED IN THE ABUTMENT AND PIER DIAPHRAGMS SHALL BE ROUGHENED FOR A DISTANCE OF 10" FROM THE BEAM END BY SANDBLASTING OR OTHER APPROVED METHODS TO PROVIDE SUITABLE BOND BETWEEN THE BEAM AND THE DIAPHRAGM IN ACCORDANCE WITH ARTICLE 2403.03, I, OF THE STANDARD SPECIFICATIONS.

ALL BEAMS ARE TO BE INCREASED IN LENGTH TO COMPENSATE FOR ELASTIC SHORTENING, CREEP AND SHRINKAGE.

IF THE STEEL DIAPHRAGM OPTION IS ALLOWED AND USED, HOLES MUST BE CAST IN THE WEB TO ACCOMMODATE THE STEEL DIAPHRAGM ATTACHMENTS AS DETAILED ON THE STEEL DIAPHRAGM DETAIL SHEET.

IF SOLE PLATE IS REQUIRED FOR BEARING, SOLE PLATE IS TO BE SET IN FORMS WHEN BEAM IS CAST AND FORMED OUT BELOW TO EXCLUDE CONCRETE AS DETAILED ON THE BEARING SHEET.

0.6" DIAMETER STRANDS STRESSED TO NOT MORE THAN 5,000 LBS. EACH MAY BE USED IN LIEU OF THE a BARS WHICH RUN THE FULL LENGTH OF THE BEAM IN THE TOP FLANGE.

REINFORCING	BAR	LIST

									₹ √ 4	17 7			
	BEAI	м	A42	$\overline{}$	A46		A50		A55	$\overline{}$		4	
		SPAN		42′-6		46′-8		50′-10		55′-0	D=2 (4	2 (
	BAR	SHAPE	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	NO.	LENGTH	2'- D=2	ω [\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	D=2
	5al		4	22'-10	4	24'-11	4	27′-0	4	29'-1		-NT D=12	
	4a2	_	2	3′-3	2	3′-3	2	3′-3	2	3′-3		M 1/-1	1 1 2
ΔΔ	4b1		40	6′-8	44	6′-8	46	6′-8	50	6′-8	ΔΔ4b1 4½	3d	462
	4b2		12	5′-0	12	5′-0	8	5′-0	8	5′-0			
	4b5		_		_		8	2′-9	12	2′-9	1'-1½ 3e	9	
	3c1		40	1′-3	44	1′-3	46	1′-3	50	1′-3		R	
**	3d	2	104	2′-8	112	2′-8	108	2′-8	116	2′-8	D=1½ N	/20	
	3е		20	1′-6	20	1′-6	18	1′-6	18	1′-6	10½ 3cl	2′-6	
											ALL DIMENSIONS ARE	4b5	
											OUT TO OUT.		
											RADIUS TO & BAR.		

D = PIN DIAMETER.

10-09 REVISION DATE

LATEST

1/√ 2/2

lowa Department of Transportation **Highway Division**

STANDARD DESIGN - 40' ROADWAY, THREE SPAN BRIDGE PRETENSIONED PRESTRESSED CONCRETE BEAM BRIDGES AUGUST, 2009

A BEAM DETAILS

H40-32-06

ΔΔ 4bl BARS TO BE EPOXY COATED.

BEAM

SPECIFICATION REFERENCES WERE

** WHERE DEFLECTING STRANDS INTERFERE WITH PLACEMENT, SOME IN-PLACE BENDING MAY BE NECESSARY.